

# THEORY OF ELASTICITY AND PLASTICITY

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# FUNDAMENTAL SOLUTIONS IN LINEAR SYSTEMS

## SUPERPOSITION PRINCIPLE

Governing equations of the linear theory of elasticity constitute a **system of linear equations** (differential and algebraic equations)

In linear system the **superposition principle** is valid – according to this principle:

An effect of a combination of causes is a respective combination of effects of all causes accounted for separately

$$\left. \begin{array}{l} L(x_1) = y_1 \\ L(x_2) = y_2 \\ \dots \\ L(x_p) = y_p \end{array} \right\} \Rightarrow L(\alpha_1 x_1 + \alpha_2 x_2 + \dots + \alpha_p x_p) = \alpha_1 y_1 + \alpha_2 y_2 + \dots + \alpha_p y_p$$

$x_i$  - **causes** – external forces (surface tractions, body forces), **kinematic excitations** (boundary conditions or inhomogeneous parts of equations)

$y_i$  - **effects** – displacement, strain, stress

$L$  - **linear operator** representing the mathematical structure of the problem – **governing equations**

# SUPERPOSITION PRINCIPLE

## PROBLEM (1)

$$\begin{cases} \sigma_{ij,j}^{(1)} + b_i^{(1)} = 0 \\ \varepsilon_{ij}^{(1)} = \frac{1}{2}(u_{i,j}^{(1)} + u_{j,i}^{(1)}) \\ \sigma_{ij}^{(1)} = 2G\varepsilon_{ij}^{(1)} + \lambda\delta_{ij}\varepsilon_{kk}^{(1)} \end{cases} \quad \begin{cases} \sigma_{ij}^{(1)} n_j = q_i^{(1)} \text{ na } S_q \\ u_i^{(1)} = \hat{u}_i^{(1)} \text{ na } S_u \end{cases}$$

## PROBLEM(2)

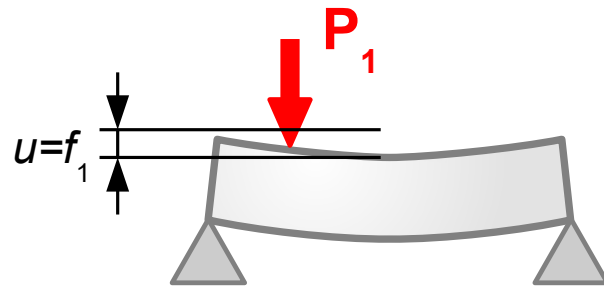
$$\begin{cases} \sigma_{ij,j}^{(2)} + b_i^{(2)} = 0 \\ \varepsilon_{ij}^{(2)} = \frac{1}{2}(u_{i,j}^{(2)} + u_{j,i}^{(2)}) \\ \sigma_{ij}^{(2)} = 2G\varepsilon_{ij}^{(2)} + \lambda\delta_{ij}\varepsilon_{kk}^{(2)} \end{cases} \quad \begin{cases} \sigma_{ij}^{(2)} n_j = q_i^{(2)} \text{ na } S_q \\ u_i^{(2)} = \hat{u}_i^{(2)} \text{ na } S_u \end{cases}$$

## PROBLEM (1+2) = PROBLEM (1) + PROBLEM (2)

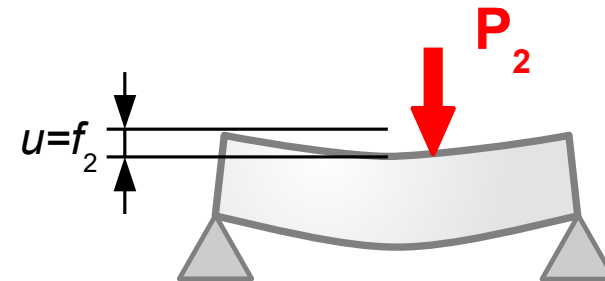
$$\begin{cases} u_i^{(1+2)} = u_i^{(1)} + u_i^{(2)} \\ \varepsilon_{ij}^{(1+2)} = \varepsilon_{ij}^{(1)} + \varepsilon_{ij}^{(2)} \\ \sigma_{ij}^{(1+2)} = \sigma_{ij}^{(1)} + \sigma_{ij}^{(2)} \\ b_i^{(1+2)} = b_i^{(1)} + b_i^{(2)} \\ \hat{u}_i^{(1+2)} = \hat{u}_i^{(1)} + \hat{u}_i^{(2)} \\ q_i^{(1+2)} = q_i^{(1)} + q_i^{(2)} \end{cases} \Rightarrow \begin{cases} \sigma_{ij,j}^{(1+2)} + b_i^{(1+2)} = 0 \\ \varepsilon_{ij}^{(1+2)} = \frac{1}{2}(u_{i,j}^{(1+2)} + u_{j,i}^{(1+2)}) \\ \sigma_{ij}^{(1+2)} = 2G\varepsilon_{ij}^{(1+2)} + \lambda\delta_{ij}\varepsilon_{kk}^{(1+2)} \end{cases} \quad \begin{cases} \sigma_{ij}^{(1+2)} n_j = q_i^{(1+2)} \text{ na } S_q \\ u_i^{(1+2)} = \hat{u}_i^{(1+2)} \text{ na } S_u \end{cases}$$

# SUPERPOSITION PRINCIPLE

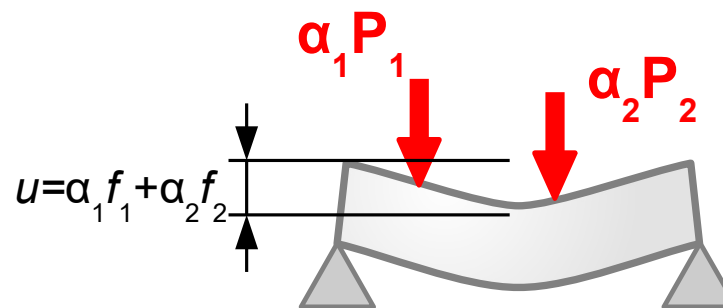
PROBLEM (1)



PROBLEM(2)

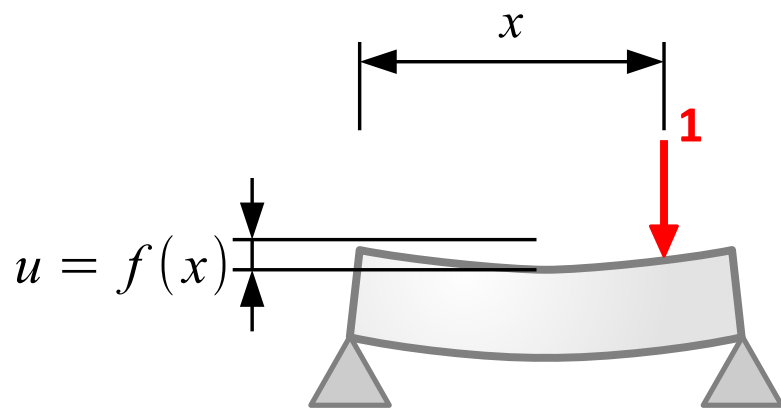


$$\text{PROBLEM } (\alpha_1 \cdot \text{„1”} + \alpha_2 \cdot \text{„2”}) = \alpha_1 \cdot \text{PROBLEM („1”)} + \alpha_1 \cdot \text{PROBLEM („2”)}$$

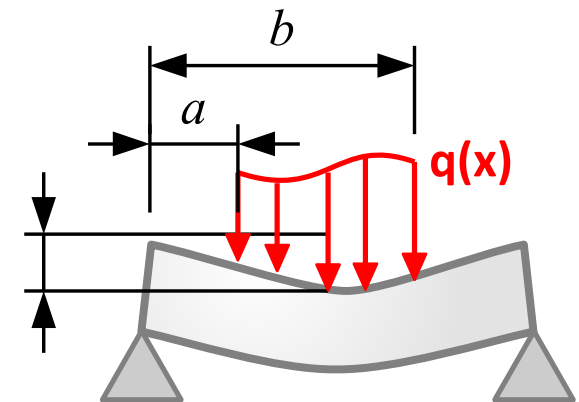


## SUPERPOSITION PRINCIPLE

In particular, we may consider a **continuous** (infinite and uncountable) **set of causes** – the effect of such a set of causes will be a **sum of an infinite and uncountable number of causes**, namely: an **integral**:



$$u = \int_a^b q(x) f(x) dx$$



## GREEN'S FUNCTIONS

If a problem is given by a **linear differential operator**  $L$ :

$$L[ u(x) ] = q(x)$$

then the function  $G$  being a **kernel of an integral operator** which is an inverse of the given differential operator will be termed the **Green's functions**. We may state that:

- **Green's function** is a solution of a differential problem in which the inhomogeneous part is given by a **Dirac's delta distribution** (“impulse function”):

$$L[ G(x, \xi) ] = \delta(\xi - x)$$

- Solution of a problem with an arbitrary inhomogeneous term may be determined as a **convolution** (an integral of a product of function – integral operator) of that inhomogeneous term with the Green's function (kernel of this operator):

$$u(x) = \int G(x, \xi) q(\xi) d\xi$$

# KELVIN'S SOLUTION



# KELVIN'S SOLUTION

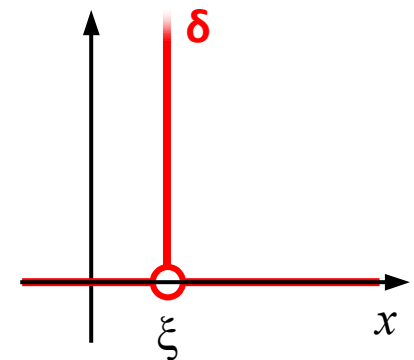
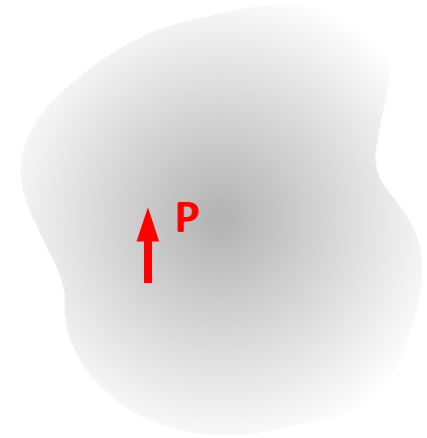
Elastic space loaded with a point force.

- whole 3-dimensional space is considered an **infinitely large elastic body**. This space may be interpreted as a **neighbourhood of an internal point** which is far away from the boundary and the neighbourhood is much smaller than the body as a whole.
- The body is made of a **homogeneous isotropic Hooke's material** characterized by elastic constants  $\lambda, G$
- **External load** is given by a **point force**, which is modelled by a **Dirac's delta distribution**:

$$\delta(x, \xi) = \begin{cases} 0 & \Leftrightarrow x \neq \xi \\ \infty & \Leftrightarrow x = \xi \end{cases}, \quad \int_{-\infty}^{\infty} \delta(x, \xi) dx = 1 \quad \Rightarrow$$

$$\Rightarrow \int_{-\infty}^{\infty} \delta(x, \xi) f(\xi) d\xi = f(x)$$

- **Boundary conditions** are prescribed in infinity – we state that **displacements, strain and stresses tend to 0 in infinity**.



## KELVIN'S SOLUTION

Elastic space loaded with a point force.

- We assume a Cartesian coordinate system with its origin in the point of application of the point force, axis  $x_3$  is oriented in the same way as the force.
- Let's make use of the **Lamé's displacement equations**:

$$G \nabla^2 u_i + (\lambda + G) u_{k,ik} + b_i = 0$$

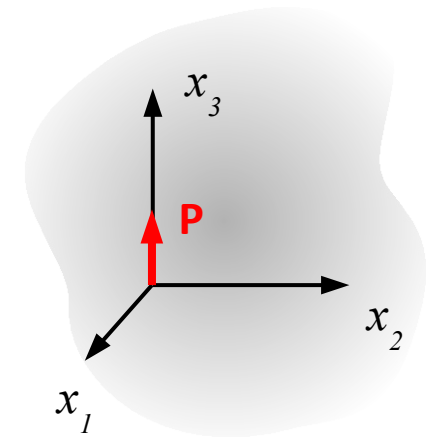
- Body forces vector**:

$$\mathbf{b} = [0; 0; P\delta_0]$$

- Boundary conditions**:

$$\lim_{R \rightarrow \infty} u_i = 0, \quad i = 1, 2, 3$$

$$\lim_{R \rightarrow \infty} u_{i,j} = 0, \quad i, j = 1, 2, 3 \quad \text{where} \quad R = \sqrt{x_1^2 + x_2^2 + x_3^2}$$



## KELVIN'S SOLUTION

We will solve the problem with the use of the **Fourier integral transform**.

In the case of a function  $f(x)$  of single independent variable, the **transform** is given by the following equation:

$$\mathcal{F}\{f(x)\}(\omega) = \hat{f}(\omega) = \int_{-\infty}^{\infty} f(x)e^{-i\omega x} dx$$

Inverse transform:

$$\mathcal{F}^{-1}\{\hat{f}(\omega)\}(x) = f(x) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \hat{f}(\omega)e^{i\omega x} dx$$

Function  $\hat{f}(\omega)$  is termed the **transform** of function  $f(x)$ , and function  $f(x)$  is termed **original function** or **inverse transform** of  $\hat{f}(\omega)$ . Original function is a function of independent variable  $x$ , while its transform is a function of variable  $\omega$ , which has an inverse physical dimension (here: 1/m).

### REMARK:

- It is a matter of convention whether sign „-“ is placed in the exponent in the definition of the transform or inverse transform. It is also a matter of convention how the factor  $(2\pi)^{-1}$  is accounted for in those definitions. Various conventions are used.

## KELVIN'S SOLUTION

In **3-dimensional** case the transform may be performed with respect to each independent variable. Each time we need to introduce a new variable in the domain of transforms. We will transform the components of the **displacement vector**.

$$\begin{aligned}\mathcal{F}\{u_i\} = \hat{u}_i &= \int_{x_1=-\infty}^{\infty} \left[ \int_{x_2=-\infty}^{\infty} \left[ \int_{x_3=-\infty}^{\infty} u_i e^{-i\omega_3 x_3} dx_3 \right] e^{-i\omega_2 x_2} dx_2 \right] e^{-i\omega_1 x_1} dx_1 = \\ &= \iiint_{-\infty}^{\infty} u_i e^{-i(x_1\omega_1 + x_2\omega_2 + x_3\omega_3)} dx_1 dx_2 dx_3\end{aligned}$$

Inverse transform:

$$u_i = \mathcal{F}^{-1}\{\hat{u}_i\} = \frac{1}{(2\pi)^3} \iiint_{-\infty}^{\infty} \hat{u}_i e^{i(x_1\omega_1 + x_2\omega_2 + x_3\omega_3)} d\omega_1 d\omega_2 d\omega_3$$

## KELVIN'S SOLUTION

Integral operator used in the definition of the Fourier transform is **linear**, so:

$$\mathcal{F} \{ \alpha_1 u_1 + \alpha_2 u_2 \} = \alpha_1 \mathcal{F} \{ u_1 \} + \alpha_2 \mathcal{F} \{ u_2 \}$$

The use of the **Fourier transform** with the **Lamé's displacement equations** gives us:

$$\mathcal{F} \{ G \nabla^2 u_i + (\lambda + G) u_{k,ik} + b_i \} = G \mathcal{F} \{ \nabla^2 u_i \} + (\lambda + G) \mathcal{F} \{ u_{k,ik} \} + \mathcal{F} \{ b_i \} = 0$$

We need to find:

- Transforms of the body forces:  $\mathcal{F} \{ b_i \} = ?$
- Transforms of the derivatives of displacements:  $\mathcal{F} \{ u_{i,jk} \} = ?$

## KELVIN'S SOLUTION

Transforms of the components of the body forces vector:

$$\mathcal{F}\{b_1\} = \mathcal{F}\{b_2\} = \mathcal{F}\{0\} = \iiint_{-\infty}^{\infty} 0 \cdot e^{-i(x_1\omega_1 + x_2\omega_2 + x_3\omega_3)} dx_1 dx_2 dx_3 = 0$$

$$\mathcal{F}\{b_3\} = \mathcal{F}\{P\delta_0\} = P \iiint_{-\infty}^{\infty} \delta_0 e^{-i(x_1\omega_1 + x_2\omega_2 + x_3\omega_3)} dx_1 dx_2 dx_3 = P e^{-i(x_1\omega_1 + x_2\omega_2 + x_3\omega_3)} \Big|_{\mathbf{x}=\mathbf{0}} = P$$

### CONCLUSIONS:

- Fourier transform of a **zero function** is a **zero function**
- Fourier transform of a **delta distribution** is a **unit function**

## KELVIN'S SOLUTION

Transforms of the derivatives of the components of the displacement vector:

Let's calculate for example  $\mathcal{F} \{u_{i,12}\}$

$$\mathcal{F} \{u_{i,12}\} = \iiint_{-\infty}^{\infty} \frac{\partial u_i}{\partial x_1 \partial x_2} e^{-i(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)} \mathrm{d}x_1 \mathrm{d}x_2 \mathrm{d}x_3$$

Let's **integrate by parts** with respect to  $x_1$

$$\mathcal{F} \{u_{i,12}\} = \iint_{-\infty}^{\infty} \left[ \left[ \frac{\partial u_i}{\partial x_2} e^{-i(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)} \right]_{x_1=-\infty}^{\infty} - \int_{x_1=-\infty}^{\infty} \frac{\partial u_i}{\partial x_2} \frac{\partial}{\partial x_1} e^{-i(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)} \mathrm{d}x_1 \right] \mathrm{d}x_2 \mathrm{d}x_3$$

Let's deal now with the **boundary term** (it would be great to get rid of it):

$$\left[ \frac{\partial u_i}{\partial x_2} e^{-i(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)} \right]_{x_1=-\infty}^{\infty}$$

## KELVIN'S SOLUTION

Transforms of the derivatives of the components of the displacement vector:

$$\left[ \frac{\partial u_i}{\partial x_2} e^{-i(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)} \right]_{x_1=-\infty}^{\infty}$$

- according to the **Euler's formula**  $e^{i\phi} = \cos \phi + i \sin \phi$  where  $\phi = -(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)$
- values of **trigonometric functions** are within interval  $\langle -1; 1 \rangle$
- as a result the term  $e^{-i(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)}$  has surely **finite value** for  $\phi \rightarrow \pm \infty$
- according to the assumed **boundary conditions** we have  $\lim_{R \rightarrow \infty} u_{i,j} = 0$
- a product of a finite value and 0 gives always 0.
- values of the boundary term for both limit cases is zero.
- finally:

$$\left[ \frac{\partial u_i}{\partial x_2} e^{-i(x_1 \omega_1 + x_2 \omega_2 + x_3 \omega_3)} \right]_{x_1=-\infty}^{\infty} = 0$$



## KELVIN'S SOLUTION

Transforms of the derivatives of the components of the displacement vector:

We have then:

$$\mathcal{F} \{u_{i,12}\} = \iint_{-\infty}^{\infty} \left[ -(-i\omega_1) \int_{x_1=-\infty}^{\infty} \frac{\partial u_i}{\partial x_2} e^{-i(x_1\omega_1+x_2\omega_2+x_3\omega_3)} dx_1 \right] dx_2 dx_3$$

$$\mathcal{F} \{u_{i,12}\} = i\omega_1 \iiint_{-\infty}^{\infty} \frac{\partial u_i}{\partial x_2} e^{-i(x_1\omega_1+x_2\omega_2+x_3\omega_3)} dx_1 dx_2 dx_3$$

Let's integrate by parts again – this time with respect to variable  $x_2$

The whole procedure and corresponding considerations is repeated. As a result we obtain:

$$\mathcal{F} \{u_{i,12}\} = (i\omega_1)(i\omega_2) \underbrace{\iiint_{-\infty}^{\infty} u_i e^{-i(x_1\omega_1+x_2\omega_2+x_3\omega_3)} dx_1 dx_2 dx_3}_{\mathcal{F} \{u_i\} = \hat{u}_i} = i^2 \omega_1 \omega_2 \hat{u}_i = -\omega_1 \omega_2 \hat{u}_i$$

In general:

$$\mathcal{F} \{u_{i,jk}\} = -\omega_j \omega_k \hat{u}_i$$

## KELVIN'S SOLUTION

The system of **Lamé's displacement equations** which is governing the **Kelvin problem**:

$$\begin{cases} G(u_{1,11} + u_{1,22} + u_{1,33}) + (\lambda + G)(u_{1,11} + u_{2,21} + u_{3,31}) = 0 \\ G(u_{2,11} + u_{2,22} + u_{2,33}) + (\lambda + G)(u_{1,12} + u_{2,22} + u_{3,32}) = 0 \\ G(u_{3,11} + u_{3,22} + u_{3,33}) + (\lambda + G)(u_{1,13} + u_{2,23} + u_{3,33}) + P\delta_0 = 0 \end{cases}$$

It is a **system of linear partial differential equations of the 2<sup>nd</sup> order**.

In the space of **transforms** the considered system takes the following form:

$$\begin{cases} -G(\omega_1^2 + \omega_2^2 + \omega_3^2)\hat{u}_1 - \omega_1(\lambda + G)(\omega_1\hat{u}_1 + \omega_2\hat{u}_2 + \omega_3\hat{u}_3) = 0 \\ -G(\omega_1^2 + \omega_2^2 + \omega_3^2)\hat{u}_2 - \omega_2(\lambda + G)(\omega_1\hat{u}_1 + \omega_2\hat{u}_2 + \omega_3\hat{u}_3) = 0 \\ -G(\omega_1^2 + \omega_2^2 + \omega_3^2)\hat{u}_3 - \omega_3(\lambda + G)(\omega_1\hat{u}_1 + \omega_2\hat{u}_2 + \omega_3\hat{u}_3) = -P \end{cases}$$

It is a **system of linear algebraic equations**. We solve it easily.

## KELVIN'S SOLUTION

The solutions of the linear system for transforms:

$$\hat{u}_1 = -\frac{P(\lambda + G)}{G(\lambda + 2G)} \frac{\omega_1 \omega_3}{(\omega_1^2 + \omega_2^2 + \omega_3^2)^2}$$

$$\hat{u}_2 = -\frac{P(\lambda + G)}{G(\lambda + 2G)} \frac{\omega_2 \omega_3}{(\omega_1^2 + \omega_2^2 + \omega_3^2)^2}$$

$$\hat{u}_3 = \frac{P}{G} \frac{1}{\omega_1^2 + \omega_2^2 + \omega_3^2} - \frac{P(\lambda + G)}{G(\lambda + 2G)} \frac{\omega_3^2}{(\omega_1^2 + \omega_2^2 + \omega_3^2)^2}$$

- we must now find **inverse transforms**.

- we're looking for  $f_1 = \mathcal{F}^{-1} \left\{ \frac{1}{\omega_1^2 + \omega_2^2 + \omega_3^2} \right\}$

- if we find  $f_2 = \mathcal{F}^{-1} \left\{ \frac{1}{(\omega_1^2 + \omega_2^2 + \omega_3^2)^2} \right\}$  then  $f_{2,ij} = \mathcal{F}^{-1} \left\{ -\frac{\omega_i \omega_j}{\omega_1^2 + \omega_2^2 + \omega_3^2} \right\}$

# KELVIN'S SOLUTION

There is a formula for inverse transform, however, calculating the resulting integrals is pretty much difficult. We may use the tables of originals and transforms:

	Function	Fourier transform unitary, ordinary frequency	Fourier transform unitary, angular frequency	Fourier transform non-unitary, angular frequency	Remarks
500	$f(\mathbf{x})$	$\hat{f}(\boldsymbol{\xi}) = \int_{\mathbb{R}^n} f(\mathbf{x}) e^{-2\pi i \mathbf{x} \cdot \boldsymbol{\xi}} d\mathbf{x}$	$\hat{f}(\boldsymbol{\omega}) = \frac{1}{(2\pi)^{\frac{n}{2}}} \int_{\mathbb{R}^n} f(\mathbf{x}) e^{-i\boldsymbol{\omega} \cdot \mathbf{x}} d\mathbf{x}$	$\hat{f}(\boldsymbol{\nu}) = \int_{\mathbb{R}^n} f(\mathbf{x}) e^{-i\mathbf{x} \cdot \boldsymbol{\nu}} d\mathbf{x}$	
501	$\chi_{[0,1]}( \mathbf{x} ) (1 -  \mathbf{x} ^2)^\delta$	$\frac{\Gamma(\delta + 1)}{\pi^\delta  \boldsymbol{\xi} ^{\frac{n}{2} + \delta}} J_{\frac{n}{2} + \delta}(2\pi \boldsymbol{\xi} )$	$2^\delta \frac{\Gamma(\delta + 1)}{ \boldsymbol{\omega} ^{\frac{n}{2} + \delta}} J_{\frac{n}{2} + \delta}( \boldsymbol{\omega} )$	$\frac{\Gamma(\delta + 1)}{\pi^\delta} \left  \frac{\boldsymbol{\nu}}{2\pi} \right ^{-\frac{n}{2} - \delta} J_{\frac{n}{2} + \delta}( \boldsymbol{\nu} )$	The function $\chi_{[0,1]}$ is the indicator function of the interval $[0, 1]$ . The function $\Gamma(x)$ is the gamma function. The function $J_{\frac{n}{2} + \delta}$ is a Bessel function of the first kind, with order $\frac{n}{2} + \delta$ . Taking $n = 2$ and $\delta = 0$ produces 402. <sup>[52]</sup>
502	$ \mathbf{x} ^{-\alpha}, \quad 0 < \text{Re } \alpha < n.$	$\frac{(2\pi)^\alpha}{c_{n,\alpha}}  \boldsymbol{\xi} ^{-(n-\alpha)}$	$\frac{(2\pi)^{\frac{n}{2}}}{c_{n,\alpha}}  \boldsymbol{\omega} ^{-(n-\alpha)}$	$\frac{(2\pi)^n}{c_{n,\alpha}}  \boldsymbol{\nu} ^{-(n-\alpha)}$	See Riesz potential where the constant is given by $c_{n,\alpha} = \pi^{\frac{n}{2}} 2^\alpha \frac{\Gamma(\frac{\alpha}{2})}{\Gamma(\frac{n-\alpha}{2})}$ . The formula also holds for all $\alpha \neq -n, -n-1, \dots$ by analytic continuation, but then the function and its Fourier transforms need to be understood as suitably regularized tempered distributions. See homogeneous distribution. <sup>[remark 6]</sup>
503	$\frac{1}{ \boldsymbol{\sigma}  (2\pi)^{\frac{n}{2}}} e^{-\frac{1}{2} \mathbf{x}^T \boldsymbol{\sigma}^{-1} \boldsymbol{\sigma}^{-1} \mathbf{x}}$	$e^{-2\pi^2 \boldsymbol{\xi}^T \boldsymbol{\sigma} \boldsymbol{\sigma}^T \boldsymbol{\xi}}$	$(2\pi)^{-\frac{n}{2}} e^{-\frac{1}{2} \boldsymbol{\omega}^T \boldsymbol{\sigma} \boldsymbol{\sigma}^T \boldsymbol{\omega}}$	$e^{-\frac{1}{2} \boldsymbol{\nu}^T \boldsymbol{\sigma} \boldsymbol{\sigma}^T \boldsymbol{\nu}}$	This is the formula for a multivariate normal distribution normalized to 1 with a mean of 0. Bold variables are vectors or matrices. Following the notation of the aforementioned page, $\boldsymbol{\Sigma} = \boldsymbol{\sigma} \boldsymbol{\sigma}^T$ and $\boldsymbol{\Sigma}^{-1} = \boldsymbol{\sigma}^{-1} \boldsymbol{\sigma}^{-1}$
504	$e^{-2\pi\alpha \mathbf{x} }$	$\frac{c_n \alpha}{(\alpha^2 +  \boldsymbol{\xi} ^2)^{\frac{n+1}{2}}}$	$\frac{c_n (2\pi)^{\frac{n+2}{2}} \alpha}{(4\pi^2 \alpha^2 +  \boldsymbol{\omega} ^2)^{\frac{n+1}{2}}}$	$\frac{c_n (2\pi)^{n+1} \alpha}{(4\pi^2 \alpha^2 +  \boldsymbol{\nu} ^2)^{\frac{n+1}{2}}}$	Here! <sup>[53]</sup> $c_n = \frac{\Gamma(\frac{n+1}{2})}{\Gamma(\frac{n}{2})}, \text{Re}(\alpha) > 0$

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$$f_1 = \frac{1}{(2\pi)^3} \iiint_{-\infty}^{\infty} \frac{e^{i(\omega_1 x_1 + \omega_2 x_2 + \omega_3 x_3)}}{\omega_1^2 + \omega_2^2 + \omega_3^2} d\omega_1 d\omega_2 d\omega_3 = \frac{1}{4\pi} \frac{1}{\sqrt{x_1^2 + x_2^2 + x_3^2}}$$

$$f_2 = \frac{1}{(2\pi)^3} \iiint_{-\infty}^{\infty} \frac{e^{i(\omega_1 x_1 + \omega_2 x_2 + \omega_3 x_3)}}{(\omega_1^2 + \omega_2^2 + \omega_3^2)^2} d\omega_1 d\omega_2 d\omega_3 = -\frac{1}{8\pi} \sqrt{x_1^2 + x_2^2 + x_3^2}$$

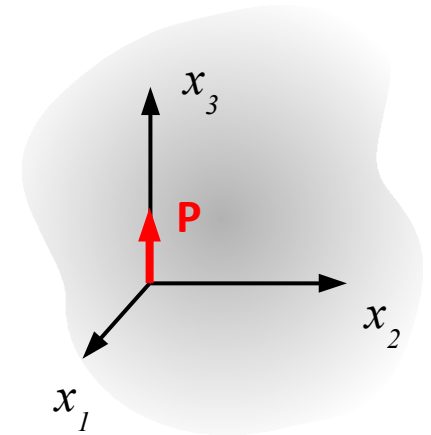
## KELVIN'S SOLUTION

Finally, the **Kelvin's solution** is given by the formulas:

$$u_1 = \frac{P(\lambda + G)}{8\pi G(\lambda + 2G)} \frac{x_1 x_3}{[x_1^2 + x_2^2 + x_3^2]^{3/2}}$$

$$u_2 = \frac{P(\lambda + G)}{8\pi G(\lambda + 2G)} \frac{x_2 x_3}{[x_1^2 + x_2^2 + x_3^2]^{3/2}}$$

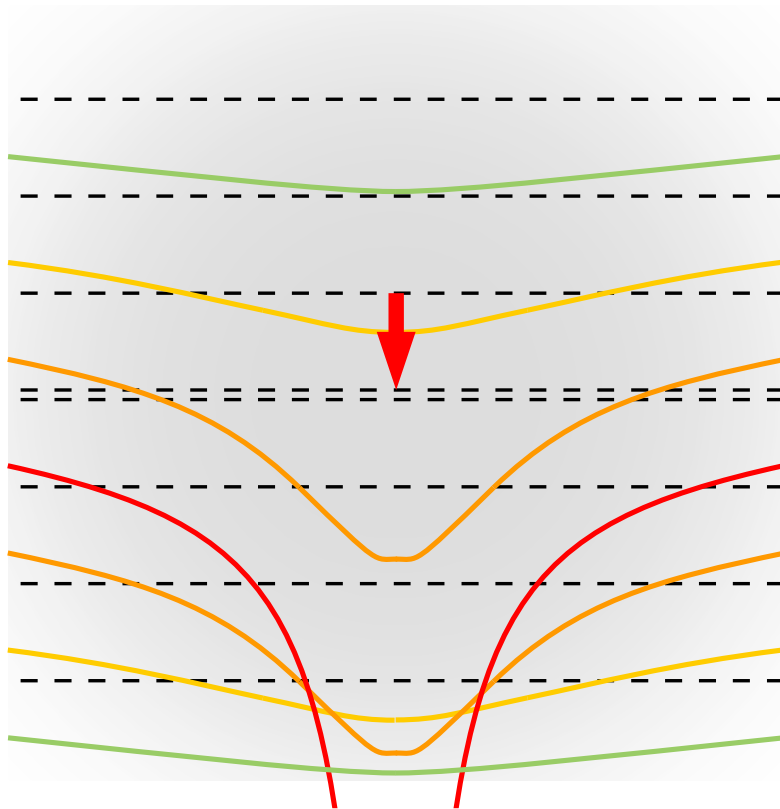
$$u_3 = \frac{P}{4\pi G} \frac{1}{\sqrt{x_1^2 + x_2^2 + x_3^2}} + \frac{P(\lambda + G)}{8\pi G(\lambda + 2G)} \frac{x_3^2}{[x_1^2 + x_2^2 + x_3^2]^{3/2}}$$



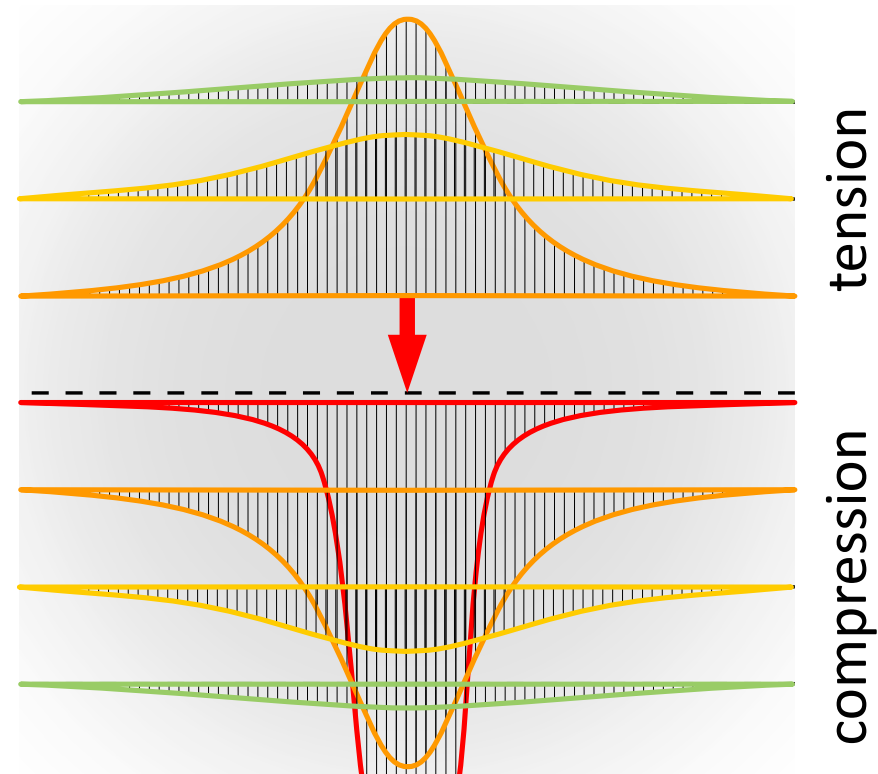
- Distribution of **strain** is determined according the **above** distribution of displacement and **kinematic relations**.
- Distribution of **stress** is determined according the **found** distribution of strain and **constitutive relations**.

# KELVIN'S SOLUTION

Kelvin's solution has a **singularity** in the point of application of the point force, namely values of the components of the displacement, strain and stress field diverge to infinity in this point:



distribution of displacement  $u_3$



distribution of stress  $\sigma_{33}$

tension

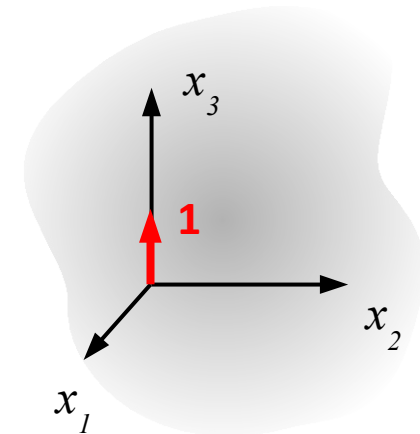
compression

## KELVIN'S SOLUTION

Kelvin's solution may be considered a **Green's function** for all problems of an elastic space loaded with any system of body forces (of given direction):

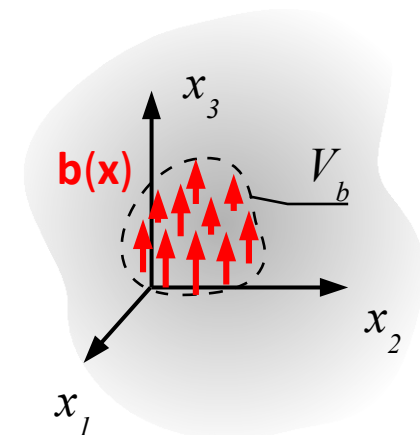
Kelvin's solution (Green's function):

$$\tilde{u}_i = \frac{1}{4\pi G} \left[ \frac{1}{[(x_n - \xi_n)(x_n - \xi_n)]^{1/2}} \delta_{i3} + \frac{1}{4(1-\nu)} \frac{(x_i - \xi_i)(x_3 - \xi_3)}{[(x_n - \xi_n)(x_n - \xi_n)]^{3/2}} \right]$$



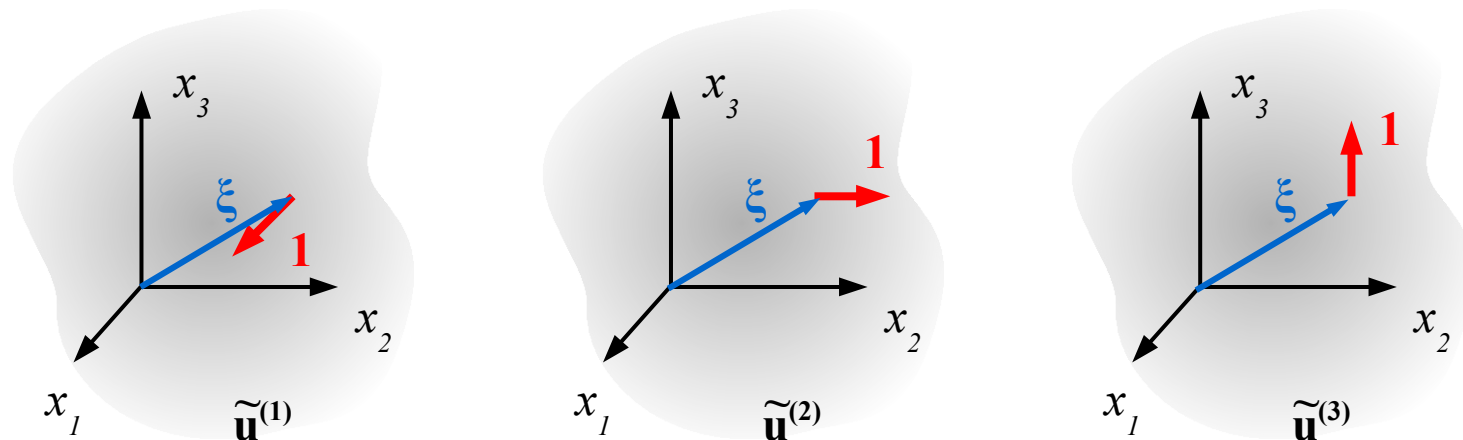
Integral of Green's function:  $\mathbf{u} = \int [\mathbf{G}(\mathbf{x} - \boldsymbol{\xi}) q(\boldsymbol{\xi})] d\boldsymbol{\xi}$

$$u_i(\mathbf{x}) = \iiint_{V_b} [\tilde{u}_i(\mathbf{x} - \boldsymbol{\xi}) b_3(\boldsymbol{\xi})] d\boldsymbol{\xi}$$



## KELVIN'S SOLUTION

- For a body forces field of varying orientation, the field is divided into 3 fields – each one of that fields has only one non-zero component, namely the one corresponding with the chosen axis of the coordinate system.
- Those fields are integrated separately and then they are added according to the **superposition principle**.



$$\tilde{u}_i^{(k)} = \frac{1}{4\pi G} \left[ \frac{1}{[(x_n - \xi_n)(x_n - \xi_n)]^{1/2}} \delta_{ik} + \frac{1}{4(1-\nu)} \frac{(x_i - \xi_i)(x_k - \xi_k)}{[(x_n - \xi_n)(x_n - \xi_n)]^{3/2}} \right]$$



# KELVIN'S SOLUTION

## REMARKS:

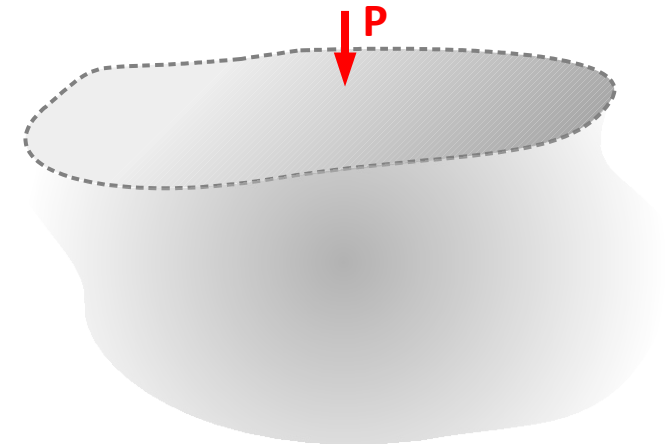
- **Kelvin's solution** may be considered a **Green's function** for all problems of an elastic space loaded with any system of body forces. In such cases **integration of the Kelvin's solution makes the singularity vanish**.
  - Solution of a problem with point load (Dirac distribution) has a singularity.
  - Solution of a problem with a continuous load doesn't have a singularity.
- For this reason this solution is termed **the fundamental solution of the linear theory of elasticity**.
- It is also used in the so called **Somigliana's formula**, which in turn is used in solving the problems concerning the **elastic bodies of finite dimensions and arbitrary shapes**.
- Somigliana's formula provides also a basis for a numerical method of solving the problems of the linear theory of elasticity, namely for the **Boundary Element Method**.

# BOUSSINESQ SOLUTION

## BOUSSINESQ SOLUTION

Elastic half-space loaded with a point force.

- Whole **elastic half-space** is considered **an infinitely large elastic body**. The half-space may be considered a **neighbourhood of a point on the boundary** of that body and the **size of that neighbourhood is much smaller than the whole body**.
- The elastic body is made of homogeneous isotropic Hooke's material characterized by elastic constants  $\lambda$ ,  $G$
- **External load** is given by a **point force** modelled by a **Dirac's delta distribution**.



## BOUSSINESQ SOLUTION

Elastic half-space loaded with a point force.

- we assume a **Cartesian coordinate** system with its **origin in the point of application of the point force**, axis  $x_3$  is oriented in the same way as the force, towards interior of the half-space.
- we will use the **Lamé's displacement equations**:

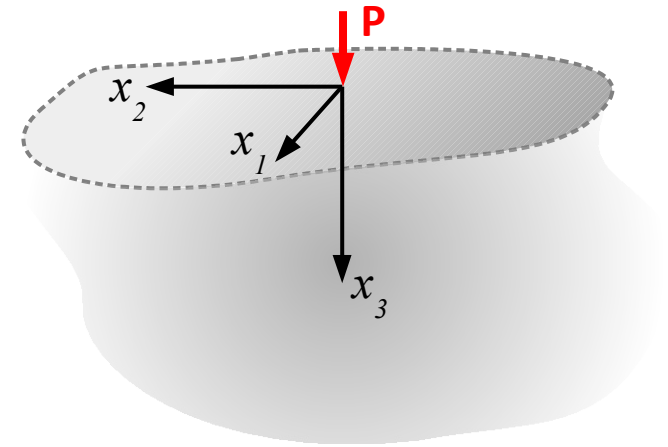
$$G \nabla^2 u_i + (\lambda + G) u_{k,ik} + b_i = 0$$

- **body forces vector**:

$$\mathbf{b} = [0; 0; 0]$$

- **surface tractions vector**:

$$\mathbf{q} = [0; 0; P \delta_0]$$



## BOUSSINESQ SOLUTION

Elastic half-space loaded with a point force.

- **Static boundary conditions:**

in plane  $x_3 = 0$  of unit normal:  $\mathbf{n} = [0; 0; -1]$

- in the point of application of the force:  $x_1 = x_2 = x_3 = 0$

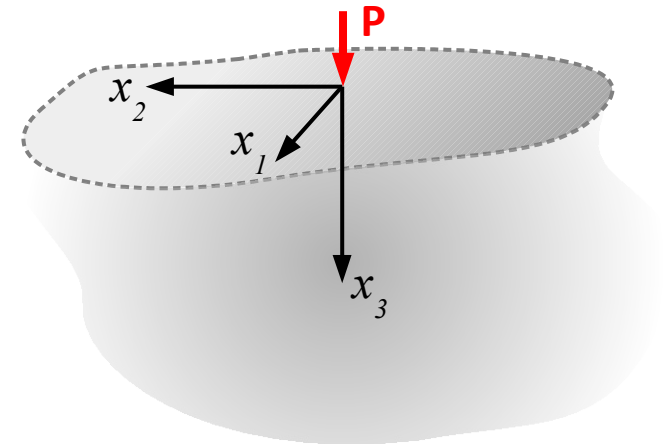
$$\sigma_{ij} n_j = q_i \quad \Rightarrow \quad \sigma_{33} = -P \delta_0, \quad \sigma_{13} = \sigma_{23} = 0$$

- otherwise:  $x_3 = 0, \quad x_1 \neq 0, \quad x_2 \neq 0$

$$\sigma_{ij} n_j = q_i \quad \Rightarrow \quad \sigma_{13} = \sigma_{23} = \sigma_{33} = 0$$

- **kinematic boundary conditions:**

$$\lim_{R \rightarrow \infty} u_i = 0, \quad \lim_{R \rightarrow \infty} u_{i,j} = 0, \quad i, j = 1, 2, 3 \quad \text{where} \quad R = \sqrt{x_1^2 + x_2^2 + x_3^2}$$



## BOUSSINESQ SOLUTION

We will look for a solution in the form of a combination of **potentials**.

Let's consider the Boussinesq potentials A and B:

- POTENTIAL A:**
  - displacement field:  $u_i^A = \frac{1}{2G} \phi_{,i}$       where  $\nabla^2 \phi = 0$
  - strain field:  $\varepsilon_{ij}^A = \frac{1}{2G} \phi_{,ij}$
  - stress field:  $\sigma_{ij}^A = \phi_{,ij}$
- POTENTIAL B:**
  - displacement field:  $u_i^B = \frac{1}{2G} [\psi_{,i} x_3 - (3-4\nu) \psi \delta_{i3}]$       where  $\nabla^2 \psi = 0$
  - strain field:  $\varepsilon_{ij}^B = \frac{1}{2G} [\psi_{,ij} x_3 - (1-2\nu)(\psi_{,j} \delta_{i3} + \psi_{,i} \delta_{j3})]$
  - stress field:  $\sigma_{ij}^B = \psi_{,ij} x_3 - (1-2\nu)(\psi_{,j} \delta_{i3} + \psi_{,i} \delta_{j3}) - 2\nu \psi_{,3} \delta_{ij}$

We assume that the **solution** is in the following form:

$$u_i(\mathbf{x}) = A u_i^A(\mathbf{x}) + B u_i^B(\mathbf{x})$$

## BOUSSINESQ SOLUTION

We assume that the solution is in the following form:

$$u_i(\mathbf{x}) = A u_i^A(\mathbf{x}) + B u_i^B(\mathbf{x})$$

REMARKS:

- we must find now such **harmonic functions** (satisfying the Laplace equations)  $\phi(\mathbf{x})$  and  $\psi(\mathbf{x})$  and such **constants**  $A$  and  $B$ , that all **boundary conditions** were satisfied.
- We're not sure if the true solutions can be represented in such a form. We cannot reject a possibility that even if such a solution is found, also other more general solutions of that problem may still exist.
- An additional assumption is that functions  $\phi(\mathbf{x})$  and  $\psi(\mathbf{x})$  are related by the following equation:

$$\psi = \phi_{,3}$$

## BOUSSINESQ SOLUTION

According to the all above stated assumption, the **stress state** may be expressed as follows:

$$\sigma_{13} = A \phi_{,13} + B (\phi_{,331} x_3 - (1 - 2\nu) \phi_{,13})$$

$$\sigma_{23} = A \phi_{,23} + B (\phi_{,332} x_3 - (1 - 2\nu) \phi_{,23})$$

$$\sigma_{33} = A \phi_{,33} + B (\phi_{,333} x_3 - 2(1 - \nu) \phi_{,33})$$

Constants  $A$  and  $B$  are chosen in such a way that in every point of the plane  $x_3 = 0$  the stress was such that:

$$\begin{cases} \sigma_{13}(x_3=0) = A \phi_{,13} - B(1 - 2\nu) \phi_{,13} = 0 \\ \sigma_{23}(x_3=0) = A \phi_{,23} - B(1 - 2\nu) \phi_{,23} = 0 \end{cases} \Rightarrow \begin{cases} A = (1 - 2\nu) \\ B = 1 \end{cases}$$

Then:

$$\sigma_{13} = \phi_{,331} x_3$$

$$\sigma_{23} = \phi_{,332} x_3$$

$$\sigma_{33} = \phi_{,333} x_3 - \phi_{,33}$$



## BOUSSINESQ SOLUTION

The boundary conditions for the shear stress are satisfied. We need to choose the harmonic functions  $\phi(\mathbf{x})$  in such a way that also the **boundary conditions for the normal stress** were satisfied, namely:

$$\text{in plane: } x_3=0 \quad \sigma_{33}(x_3=0) = -\phi_{,33} = \begin{cases} 0 & \Leftrightarrow x_1 \neq 0, x_2 \neq 0 \\ -P\delta & \Leftrightarrow x_1 = x_2 = 0 \end{cases}$$

The above conditions provide the boundary conditions for the **Laplace equation** which must be satisfied by the potential:

$$\nabla^2 \phi = \phi_{,11} + \phi_{,22} + \phi_{,33} = 0$$

We may perform the **Fourier transform** with respect to the variables  $x_1, x_2$

$$\mathcal{F}\{\phi_{,11}\} = -\omega_1^2 \hat{\phi}$$

$$\mathcal{F}\{\phi_{,22}\} = -\omega_2^2 \hat{\phi}$$

$$\mathcal{F}\{\phi_{,33}\} = \frac{\partial^2}{\partial x_3^2} \hat{\phi}$$

$$\Rightarrow \mathcal{F}\{\nabla^2 \phi\} = \frac{\partial^2}{\partial x_3^2} \hat{\phi} - (\omega_1^2 + \omega_2^2) \hat{\phi} = 0$$

where  $\phi = \phi(x_1, x_2, x_3)$   
 $\hat{\phi} = \hat{\phi}(\omega_1, \omega_2, x_3)$

## BOUSSINESQ SOLUTION

Obtained equation is an **ordinary differential equation** with respect to variable  $x_3$  - it is a **2<sup>nd</sup> order linear equation** with **constant coefficients**:

$$\frac{\partial^2}{\partial x_3^2} \hat{\phi} - (\omega_1^2 + \omega_2^2) \hat{\phi} = 0$$

The **solution** can be found easily:

$$\hat{\phi}(\omega_1, \omega_2, x_3) = C_1 e^{(\omega_1^2 + \omega_2^2)x_3} + C_2 e^{-(\omega_1^2 + \omega_2^2)x_3}$$

**Constants of integration** are chosen in such a way that the **boundary conditions** were satisfied:

- one of those conditions is that  $\lim_{R \rightarrow \infty} u_i = 0$ , in particular  $\lim_{x_3 \rightarrow \infty} u_3 = 0$ ,

$$u_3 = \frac{1-2\nu}{2G} \phi_{,3} = \frac{1-2\nu}{2G} \left[ C_1 (\omega_1^2 + \omega_2^2) e^{(\omega_1^2 + \omega_2^2)x_3} - C_2 (\omega_1^2 + \omega_2^2) e^{-(\omega_1^2 + \omega_2^2)x_3} \right] = 0$$

- it is possible only for  $C_1 = 0$

## BOUSSINESQ SOLUTION

The other constant will be determined according to the **static boundary condition** in the point of application of the point force:

$$\sigma_{33}|_{x_3=0} = -\phi_{,33}|_{x_3=0} = -P \delta_0$$

In the **space of transforms** (with respect to variables  $x_1, x_2$  )

$$\mathcal{F} \{ \phi_{,33}|_{x_3=0} \} = P \mathcal{F} \{ \delta_0 \} \quad \Rightarrow \quad \left. \frac{\partial^2 \hat{\phi}}{\partial x_3^2} \right|_{x_3=0} = P$$

$$\left. \frac{\partial^2 \hat{\phi}}{\partial x_3^2} \right|_{x_3=0} = \left. \frac{\partial^2}{\partial x_3^2} \left[ C_2 e^{-(\omega_1^2 + \omega_2^2)x_3} \right] \right|_{x_3=0} = C_2 (\omega_1^2 + \omega_2^2)^2 e^{-(\omega_1^2 + \omega_2^2)x_3} \Big|_{x_3=0} = C_2 (\omega_1^2 + \omega_2^2)^2 = P$$

$$C_2 = \frac{P}{(\omega_1^2 + \omega_2^2)^2}$$

## BOUSSINESQ SOLUTION

Transform of the potential:

$$\hat{\phi} = \frac{P}{(\omega_1^2 + \omega_2^2)^2} e^{-(\omega_1^2 + \omega_2^2)x_3}$$

Inverse transform must be found now:

$$\phi = \frac{P}{(2\pi)^2} \iint_{-\infty}^{\infty} \frac{e^{(\omega_1^2 + \omega_2^2)x_3} \cdot e^{i(\omega_1 x_1 + \omega_2 x_2)}}{(\omega_1^2 + \omega_2^2)^2} d\omega_1 d\omega_2$$

It's again worth using the tables of originals and transforms. **Original of the potential** is equal:

$$\phi = -\frac{P}{2\pi} \ln\left(\sqrt{x_1^2 + x_2^2} + x_3 + x_3\right)$$

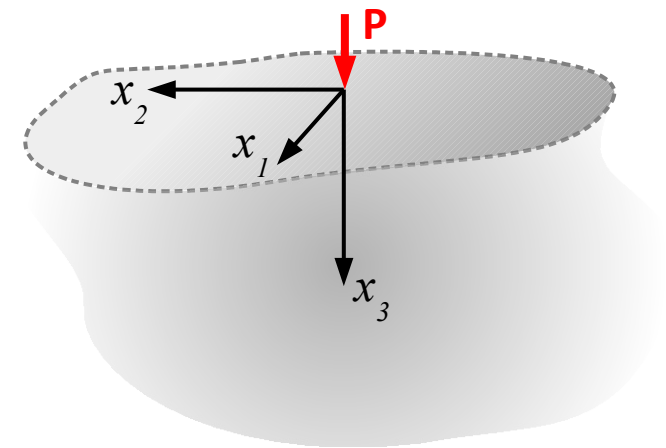
## BOUSSINESQ SOLUTION

Distribution of **displacements** given by the **Boussinesq solution**:

$$u_1 = \frac{P x_1}{4 \pi G} \left[ \frac{x_3}{R^3} - \frac{1-2\nu}{R(R+x_3)} \right]$$

$$u_2 = \frac{P x_2}{4 \pi G} \left[ \frac{x_3}{R^3} - \frac{1-2\nu}{R(R+x_3)} \right]$$

$$u_3 = \frac{P}{4 \pi G} \left[ \frac{x_3^2}{R^3} + (3-4\nu) \frac{1}{R} - \frac{1-2\nu}{R+x_3} \left( \frac{x_3}{R} + 1 \right) \right]$$

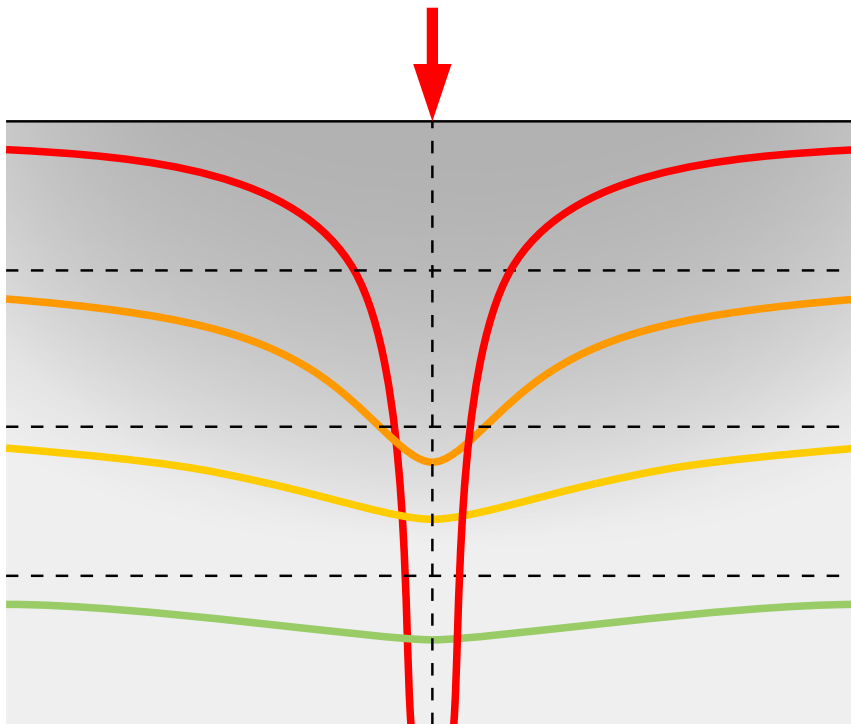


In general: 
$$u_i = \frac{1}{4 \pi G} \left[ \frac{x_3 x_i}{R^3} + (3-4\nu) \frac{\delta_{i3}}{R} - \frac{1-2\nu}{R+x_3} \left( \frac{x_i}{R} + \delta_{i3} \right) \right]$$

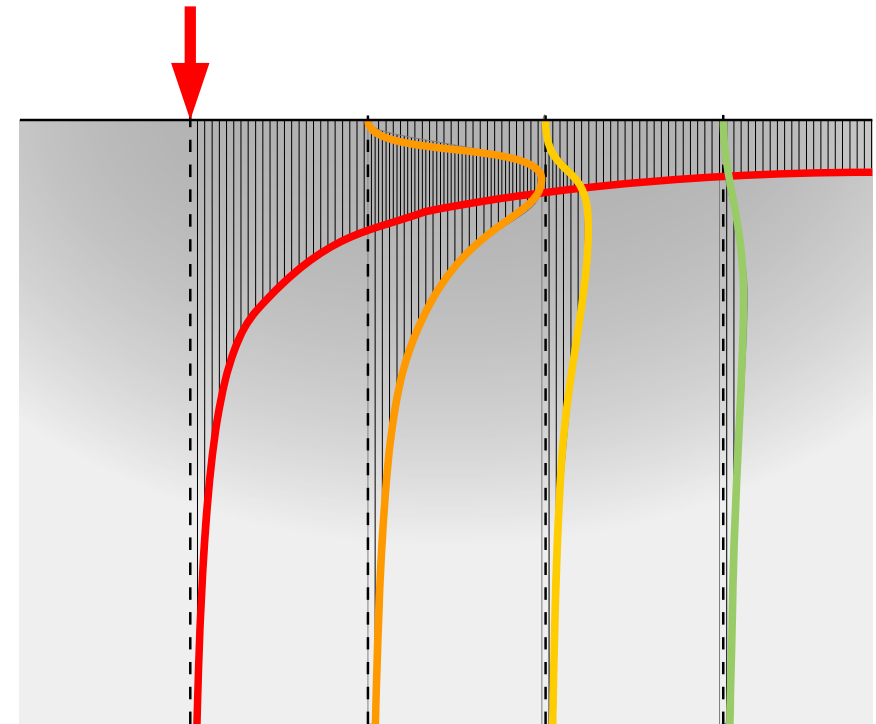
Strain is found according to kinematic relations, and stress is found according to constitutive relations.

## BOUSSINESQ SOLUTION

The Boussinesq solution has a **singularity** in the point of application of the point force, namely values of the components of the displacement, strain and stress field diverge to infinity in this point:



distribution of displacement  $u_3$



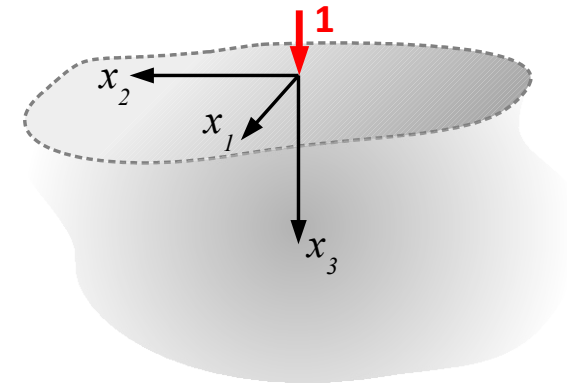
distribution of stress  $\sigma_{33}$

## BOUSSINESQ SOLUTION

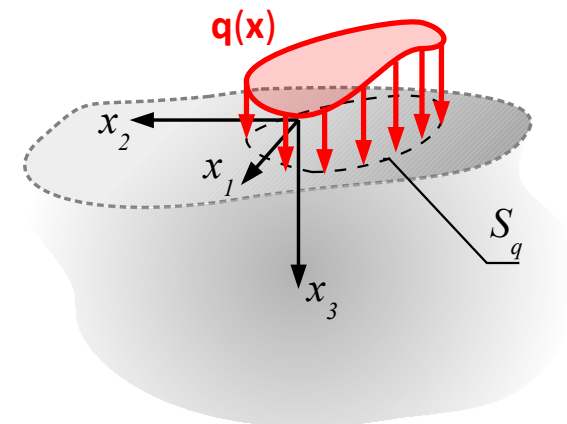
**Boussinesq solution** may be used as a **Green's function** in order to find displacements, strains and stresses in an **elastic half-space loaded with any continuous surface tractions field** which is perpendicular to the plane  $x_3 = 0$ . For **continuous and bounded** loads there is **no singularity**.

**Boussinesq solution** (Green's function):

$$\tilde{u}_i(x_1, x_2, x_3) = \frac{1}{4\pi G} \left[ \frac{x_3 x_i}{R^3} + (3 - 4\nu) \frac{\delta_{i3}}{R} - \frac{1 - 2\nu}{R + x_3} \left( \frac{x_i}{R} + \delta_{i3} \right) \right]$$



Integral of the Green's function:  $\mathbf{u} = \int [\mathbf{G}(\mathbf{x} - \boldsymbol{\xi}) q(\boldsymbol{\xi})] d\boldsymbol{\xi}$

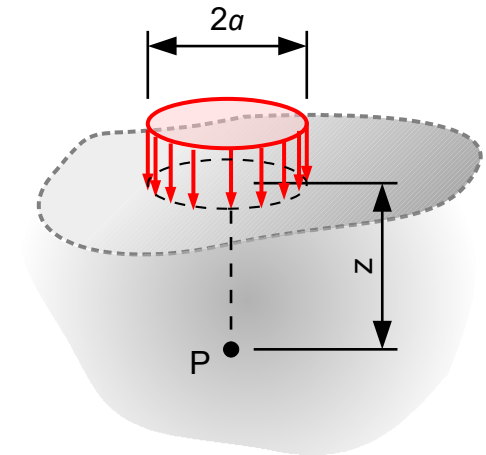


$$u_i = \iint_{S_q} [\tilde{u}_i(x_1 - \xi_1, x_2 - \xi_2, x_3) q(\xi_1, \xi_2)] d\xi_1 d\xi_2$$

## BOUSSINESQ SOLUTION

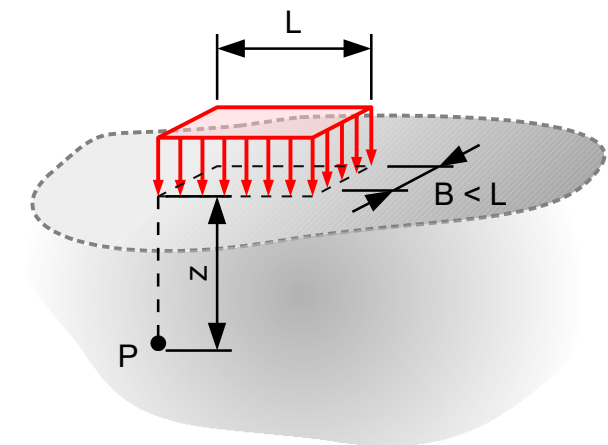
Stress distribution under the centre of a uniformly loaded circular area:

$$\sigma_{zz}(z) = q \left[ \frac{1}{\left(1 + \left(\frac{a}{z}\right)^2\right)^{3/2}} - 1 \right]$$



Stress distribution under the corner of a uniformly loaded rectangular area:

$$\sigma_{zz} = \frac{q}{4\pi} \left[ \frac{2 \frac{BL}{z^2} \sqrt{\left(\frac{B}{z}\right)^2 + \left(\frac{L}{z}\right)^2 + 1}}{\left(\frac{B}{z}\right)^2 + \left(\frac{L}{z}\right)^2 + \left(\frac{BL}{z^2}\right)^2 + 1} \cdot \frac{\left(\frac{B}{z}\right)^2 + \left(\frac{L}{z}\right)^2 + 2}{\left(\frac{B}{z}\right)^2 + \left(\frac{L}{z}\right)^2 + 1} + \operatorname{arctg} \left( \frac{2 \frac{BL}{z^2} \sqrt{\left(\frac{B}{z}\right)^2 + \left(\frac{L}{z}\right)^2 + 1}}{\left(\frac{B}{z}\right)^2 + \left(\frac{L}{z}\right)^2 - \left(\frac{BL}{z^2}\right)^2 + 1} \right) \right]$$



Boussinesq solution is used in **soil mechanics** and consequently also in **geotechnical standards**.



**THANK YOU FOR YOUR ATTENTION**