# Strength of Materials

# 4. Composed bending, eccentric tension, cross-section core

#### Composed bending

(y, z) – principal central inertia axes

cross-sectional forces:

stress distribution:  $\sigma_x = \frac{N}{A} + \frac{M_y}{I_y}z - \frac{M_z}{I_z}y$  surface equation stress solid is limited by two surfaces: the cross-section surface and stress surface these surfaces intersect at the neutral axis, where  $\sigma_x = 0$  $z = -\frac{N}{A}\frac{l_y}{M_y} - \frac{M_z}{l_z}\frac{l_y}{M_y}y$ , the neutral axis is a straight line it doesn't pass through the cross-section centroid

#### Composed bending – limit stress state



where is the maximum value of stress? at the points the furthest from the neutral axis, of course! does any method exist to check the calculated position of the neutral axis?



usually the influence of axial force is not predominant

### Composed bending – limit stress state cont.

P – the most distant point from the neutral axis

$$\sigma_x = \frac{N}{A} + \frac{M_y}{I_y} z_P - \frac{M_z}{I_z} y_P, \qquad \max|\sigma_x| \le R$$

#### **Calculations efficiency**

search the most distant point or just check the stress at some points?

Problem:

A rectangular cross-section 
$$2a \times a$$
,  $A = 2a^2$ ,  $I_y = \frac{2}{3}a^4$ ,  $I_z = \frac{1}{6}a^4$ ,  $y_p = \frac{a}{2}$ ,  $z_p = a^2$ ,

The unknown parameter occurs several times in the above formula

solve the cubical equation or use trial and error method

analytically: Cardano's formulae; on computer: fast plot of the function for roots assessment on calculator: break the formula up, next trial and error

$$\frac{N}{2a^2} \le R \to a_1 \qquad \qquad \frac{3M_y}{2a^3} \le R \to a_2 \qquad \qquad \left| -\frac{3M_z}{a^3} \right| \le R \to a_3 \qquad \qquad \to a \approx \max(a_1, a_2, a_3) + \delta$$

Keep in mind: the (very) precise values are not needed !

#### Eccentric tension - definition









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Neutral axis touching cross-section contour



side view

stress distribution cross-section view



side view

stress distribution cross-section view

#### Eccentric tension – resultant force position

medieval cathedrals – aisle height: Wien (Austria) – 22.4 m Burgos (Spain) – 25 m Mariacki (Cracow) – 28 m York Minster (York) – 31 m Notre Dame (Paris) – 32.5 m Notre Dame (Chartres) – 36.55 m Notre Dame (Reims) – 38 m St. Peter & SM (Koln) – 43.35 m Santa Maria (Milan) – 45 m Santa Maria di Fiore – 43 m (70 m with dome)





#### Eccentric tension - example



cut-out: a) from one side, b) from both sides dimensions: width h = 10 mm, thickness b = 1 mm, cut-out width d = 3 mm 1) calculate max $\sigma_x$  if N = 1 kN 2) determine the bearing capacity if R = 350 MPa Solution

1) normal stress in the middle cross-section

a) 
$$\sigma_x = \frac{N}{b(h-d)} + \frac{N \cdot \frac{d}{2} \cdot 12}{b(h-d)^3} \cdot \frac{h-d}{2} = \frac{1 \cdot 10^3}{0.001 \cdot 0.007} + \frac{1 \cdot 10^3 \cdot 0.0015 \cdot 12}{0.001 \cdot 0.007^3} \cdot 0.0035 = 327 \text{ MPa}$$
  
b)  $\sigma_x = \frac{N}{b \cdot (h-d)} = \frac{1 \cdot 10^3}{0.001 \cdot 0.004} = 250 \text{ MPa}$   
2) bearing capacity  
a)  $N = \frac{R}{\frac{1}{b(h-d)} + \frac{6d}{b(h-d)^3} \cdot \frac{h-d}{2}} = \frac{350 \cdot 10^6}{\frac{1}{0.001 \cdot 0.007} + \frac{6 \cdot 0.003}{0.001 \cdot 0.007^3} \cdot 0.0035} = 1.07 \text{ kN}$   
b)  $N = R \cdot b \cdot (h - d) = 350 \cdot 10^6 \cdot 0.001 \cdot 0.004 = 1.4 \text{ kN}$ 

Conclusion: the case b) is more favorable

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 $1 + \frac{zz_N}{i_y^2} + \frac{yy_N}{i_z^2} = 0$  Dual interpretation of neutral axis equation

(y, z) – regards the neutral axis  $(y_N, z_N)$  – regards the acting force

if the point of the force position is known – we get the neutral axis equation

point (force)

line (n.a.)

if the point of the neutral axis is known – we get the line of the applied force action

point (n.a.) line (force)

#### Eccentric tension – cross-section core

ceramic materials: very good strength in compression (breaking height in compression: steel 5.6 km, granite over 8 km) very poor strength in tension, so: unidirectional material behavior

the part of the cross-section in tension is unused usually this part is visibly cracked it is susceptible to humidity intrusion and subsequent bursting due to freezing soil under footing is not consolidated

it is highly desirable the whole cross-section works; the problem is: where the force should be applied to avoid tension stress

this happen when the neutral axis will be outside of the cross-section or at least touching the cross-section outlined (the cross-section outline: the smallest convex figure containing entire cross-section)

a cross-section core: a locus of the eccentric force position causing a stress of one sign in the cross-section

The cross-section core is limited from without by a core curve and corresponds to the all neutral axes touching the cross-section (cross-section outlined)

#### Cross-section core - example





#### Cross-section core – example cont.

The solution from the famous program "section":

Pole powierzchni: 84.0 Środek ciężkości: (4.0, 7.2222) Centralne momenty bezwładności: Iy=861.19, Iz=470.67, Iyz=0.0 Główne centralne momenty bezwładności: I1=861.19, I2=470.67 Kat głównych centralnych momentów bezwładności: 0.0 rad, 0.0 Macierz przekształcenia do współrzędnych głównych centralnych: 0.0, -4.0 1.0 0.0. -7.2222 1.0, 0. 0, Wskaźniki wytrzymałości na zginanie (sprężyste): W1=119.24, W2=117.67 Punkty rdzenia przekroju (współrzędne wyjściowe / główne centralne): (4.0, 8.6418) / (0.0, 1.4195) (2.5992, 7.2222) / (-1.4008, 0.0) (3.4982, 5.8451) / (-0.50178, -1.3772) (4.0, 5.7096) / (0.0, -1.5126) (4.5018, 5.8451) / (0.50178, -1.3772) (5.4008, 7.2222) / (1.4008, 0.0)





the cross-section core is always convex it surrounds the centroid when the cross-section is n-polygon, the core is an n-polygon also never exceeds the cross-section outline the core is a purely geometric feature of the cross-section

## That's all, folks!